Confidential



7th Framework Programme

FP7-SEC-2012.4.3-1

Next Generation Damage and Post-Crisis Needs Assessment Tool for Reconstruction and Recovery Planning Capability Project

Report on the analyses of component testing and the algorithms correlating tag position before and after blast event

Deliverable No.	D3.1		
Workpackage No.	WP3	Workpackage Title	Damage, Loss and Needs Assessment
Author(s)	Håkan Hansson, Anneli Ehlerding, Hamid Rabia, Niklas Johansson		
Status	Consortium reviewed		
Version No.	V 1.00		
File Name	"RECONASS_D3.1_Report_on_the_analyses_of_component_testing_and_the_algorith ms_correlating_tag_position_before_and_after_blast_event_v1.00"		
Delivery Date	28 09, 2015		
Project First Start and Duration	Dec. 1, 2013; 42 r	months	



"This project has received funding from the European Union's Seventh Framework Programme for research, technological development and demonstration under grant agreement no [312718]"

DOCUMENT CONTROL PAGE

Title	Report on the analyses of component testing and the algorithms correlating	
	tag position before and after blast event	
Authors	Name	Partner
	Håkan Hansson	FOI
	Anneli Ehlerding	FOI
	Hamid Rabia	FOI
	Niklas Johansson	FOI
Contributors	Name	Partner
Peer Reviewers	Name	Partner
	Dimitris Bairaktaris	DBA
	Corrado Sanna	TECNIC
Format	Text-MS Word	
Language	en-UK	
Work Package	WP3	
Deliverable Number	D3.1	
Due Date of Delivery	30/09/2015	
Actual Date of Delivery	28/09/2015	
Dissemination Level	PP	
Rights	RECONASS Consortium	
Audience	D public	
	⊠ restricted	
	internal	
Revision	(none)	
Edited by		
Status	🗌 draft	
	🖂 Consortium reviewed	
	WP leader accepted	
	Project coordinator accepted	

REVISION LOG

Version	Date	Reason	Name and Company
V 0.01	04/09/2015	First draft	Håkan Hansson (FOI)
V 0.02	24/09/2015	Consortium reviewed, and FOI internally reviewed	Håkan Hansson (FOI), Dimitris Bairaktaris (DBA), Corrado Sanna (TECNIC)
V 1.00	28/09/2015	Document finalisation	Evangelos Sdongos (ICCS), Stephanos Camarinopoulos (RISA)

TABLE OF CONTENTS

DOCUME	ENT CONTROL PAGE	2
REVISIO	N LOG	3
TABLE C	F CONTENTS	4
LIST OF	FIGURES	6
LIST OF	TABLES	8
ABBREV	IATIONS AND ACRONYMS	9
EXECUT	IVE SUMMARY	0
1 INT	RODUCTION	1
1.1	GENERAL PROJECT OVERVIEW	1
1.2	FE ANALYSIS OVERVIEW.	1
13	REPORT OVERVIEW 1	1
2 FIN		י 2
2 1 11		2
2.1	AIR BLAST ANALYSIS MODEL SETUP	3 1
Z.I. 2.1	Air blast analyse of high explosive modelling for slab tests	4
Ζ.Ι.		0
2.2	STRUCTURAL RESPONSE ANALYSIS MODEL SETUP	6
2.2	1 Structural response analysis for two span concrete slabs	6
2.2.	2 Structural response analysis for multi-node structure	8
3 RE	SULT	0
3.1	AIR BLAST AND STRUCTURAL ANALYSES FOR TWO SPAN SLABS	0
3.1	1 Air blast pressure	0
3.1	2 Structural deformation 2	6
3.2	AIR BLAST AND STRUCTURAL ANALYSES FOR MULTI-NODE STRUCTURE	9
3.2	.1 Air blast pressure	9
3.2.	.2 Structural deformation	5
4 DIS	CUSSION	1
4.1	AIR BLAST ANALYSES	1
4.2	STRUCTURAL RESPONSE	1

4	.3	METHODOLOGY	42
5	CAL	CULATION OF DISPLACEMENT VECTORS	43
6	CON	NCLUSIONS	48
REF	EREN	ICES	49

LIST OF FIGURES

Figure 2.1	Layout of slab positioning on concrete supports and charge arrangement1	4
Figure 2.2	Still images from the high speed videos (Berglund et al., 2015)	4
Figure 2.3	Placement of the 8.4 kg RDX/TNT charge at the distance of 4.5 m from the multi-node concrete	
	frame, and a video frame from a high speed video recording taken during this test (Berglund et al.,	,
	2015) 1	5
Figure 2.4	The 3D air blast model with reflecting surfaces shown (a), and half symmetry model with outflow boundaries shown (b).	6
Figure 2.5	Slab fixed to concrete supports prior to test (Berglund et al., 2015).	6
Figure 2.6	Lavout of the slabs with reinforcement location (Berglund et al., 2015)	7
Figure 2.7	The half symmetry solid model of concrete slab (a), and the half symmetry simplified mesh for the	
	reinforcement (b)	8
Figure 2.8	Lavout of the frames with reinforcement location (Berglund et al., 2015).	8
Figure 2.9	The front side of the concrete frame structure, with its three pressure gauges P1 to P3 are shown	
0	(Berglund et al., 2015)	9
Figure 2.10	Finite element model for concrete frame, with solid elements for the concrete and beam elements	
0	for the reinforcement bars is shown1	9
Figure 3.1	Calculated reflected pressure and impulse density approximately at P1 (red) and P2 (blue)	
U U	locations for 2.0 kg HE charge. Location according to Figure 2.6	21
Figure 3.2	Measured reflected pressure and impulse density at the centre points between the supports. Slab	
·	No. 1 with 2.0 kg HE (Berglund et al., 2015). Location according to Figure 2.6	21
Figure 3.3	Calculated reflected pressure (a) and impulse density (b) approximately at P1 (red) and P2 (blue)	
-	locations for 3.6 kg HE charge. Location according to Figure 2.6	21
Figure 3.4	Measured reflected pressure and impulse density at the centre points between the supports. Slab	
	No. 2 with 3.6 kg HE (Berglund et al., 2015)	22
Figure 3.5	Measured reflected pressure at the centre points between the supports. Slab No. 4 with 3.6 kg HE	-
	(Berglund et al., 2015)	22
Figure 3.6	Calculated reflected pressure and impulse density approximately at P1 (red) and P2 (blue)	
	locations for 4.8 kg HE charge 2	22
Figure 3.7	Measured reflected pressure at the centre points between the supports. Slab No. 3 with 4.8 kg HE	
	(Berglund et al., 2015)	23
Figure 3.8	Calculated reflected pressure at varying horizontal distance from the 2.0 kg charge	23
Figure 3.9	Calculated reflected pressure at varying horizontal distance from the 3.6 kg charge	23
Figure 3.10	Calculated reflected pressure at varying horizontal distance from the 4.8 kg charge	24
Figure 3.11	Shock wave propagation shown from the 3.6 kg cylindrical HE charge at 2.5 m height. Plotted	
	variable is absolute pressure in kPa, at 0.3, 0.5, 0.7, 0.9, 1.1, 1.3, 1.5 and 1.7 ms	25
Figure 3.12	Stress-strain relationship for reinforcement	26
Figure 3.13	Calculated deflection for slab No. 1 with 2.0 kg HE charge	26
Figure 3.14	Calculated deflection for slab No. 2 with 3.6 kg HE charge	27
Figure 3.15	Calculated deflection for slab No. 3 with 4.8 kg HE charge	27
Figure 3.16	Damage plot from FE analysis of slab No. 1 with 2.0 kg HE charge	28
Figure 3.17	Damage plot from FE analysis of slabs No. 2 and 4 with 3.6 kg HE charge	28
Figure 3.18	Damage plot from FE analysis of slab No. 3 with 4.8 kg HE charge	28
Figure 3.19	Axial stresses in reinforcement from FE analysis of No. 3 with 4.8 kg HE charge. Scale in GPa 2	28
⊢igure 3.20	Axial strain in reinforcement from FE analysis of No. 3 with 4.8 kg HE charge	28
Figure 3.21	Post-test photo of the reinforced concrete slabs, a) slab No. 1, 2.0 kg HE, b-c) slabs No. 2 and 4,	
F: 0.00	3.6 kg HE, d) slab No. 3, 4.8 kg HE (Berglund et al., 2015)2	<u>'9</u>
Figure 3.22	Pressure distribution acting on the test object 3.7, 3.9, 4.1, 4.3, 4.5 and 4.7 ms after detonation will	th
	the charge placed 4.5 m from the front wall. Plotted variable is absolute pressure in kPa	50

Figure 3.23	Pressure distribution acting on the test object 2.3, 2.5, 2.7, 2.9, 3.1 and 3.3 ms after detonation we the charge placed 3.5 m from the front wall. Plotted variable is absolute pressure in kPa.	vith 31
Figure 3.24	Pressure distributions from a ground surface air blast at the distance 3.5 m from the front wall at	20
Figure 2.05	2.0, 5.0, 5.2, 5.4, 5.0 and 5.0 ms aller detonation. Protect 2.5 m from the well of 2.9 and 2.0 ms offer	JΖ
Figure 5.25	Pressure distributions from a ground surface an blast 5.5 m from the wall at 2.6 and 5.0 ms after the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height and 4.5 m from the surface at 0.75 m height at 0.75 m	1 - 1
	detonation (a), and pressure distributions for the charge at 0.75 m height and 4.5 m from the wai	1 81
E :	2.3 and 2.5 ms after detonation (b). Plotted variable is absolute pressure in KPa.	33
Figure 3.26	Calculated reflected pressure (a) and impulse density (b) at the centre of the front wall of test fra	me
E: 0.07	for different norizontal distances to the centre of RDX/TNT charge	34
Figure 3.27	Measured reflected pressure at the centre of the front wall of test frame for test frame No. 1 with	25
E :	charge at 4.5 m from front wall (Berglund et al., 2015).	35
Figure 3.28	Measured reflected pressure at the centre of the front wall of test frame for test frame No. 2 with	25
E :	charge at 3.5 m from front wall (Berglund et al., 2015).	35
Figure 3.29	Deformation plot from FE analysis of frame No. 1 with the distance 4.5 m from the RDX/INI	~~
E: 0.00	charge, mid points at front and back walls.	36
Figure 3.30	Deformation from FE analysis of frame No. 2 with the distance 3.5 m from the RDX/INT charge,	20
E: 0.04	mid points at front and back walls.	36
Figure 3.31	Damage plots from FE analysis of frame No. 1 with the distance 4.5 m from the RDX/INT charge	Э.
E :	Dest test shate of multi mode commute structure Ne. 4	31
Figure 3.32	Post-test photo of multi-hode concrete structure No. 1.	31
Figure 3.33	Axial stresses in reinforcement from FE analysis of frame No. 1 with the distance 4.5 m from the	20
E :	RDX/INI charge. Scale in GPa	38
Figure 3.34	Axial strain reinforcement from FE analysis of frame No. 1 with the distance 4.5 m from the	20
E: 0.05	RDX/INI charge. The close up to the right show lower part of the front wall	38
Figure 3.35	Damage plot from FE analysis of frame No. 2 with the distance 3.5 m from the RDX/TNT charge	. 39
Figure 3.36	Post-test photo of multi-hode concrete structure No. 2.	39
Figure 3.37	Axial stresses in reinforcement from FE analysis of frame No. 2 with the distance 3.5 m from the	40
E :	RDX/INI charge. Scale in GPa	40
Figure 3.38	Axial strain in reinforcement from FE analysis of frame No. 2 with the distance 3.5 m from the	40
E' E 4	RDX/INI charge. The close up to the right show lower part of the front wall	40
Figure 5.1	Web service request message structure diagram	44
Figure 5.2	vveb service response message structure diagram.	45
Figure 5.3	GUI for monitoring web service communication.	45
Figure 5.4	Web service request example message.	46
Figure 5.5	Web service response example message.	47

LIST OF TABLES

Table 2.1	Material parameters for the JWL high explosive equation of state	13
Table 3.1	Measured pressure and impulse density in centre of the slabs free spans (Berglund et al., 201	5). 20
Table 3.2	Calculated deformation in the centre of the spans of the slabs	27
Table 3.3	Deformation in the centre of the spans of the slabs (Berglund et al., 2015).	27
Table 3.4	Maximum pressure and impulse density at the centre of the frames for gauge location P2	33
Table 3.5	Deformation of the midpoint on the front and back wall.	37

ABBREVIATIONS AND ACRONYMS

ABBREVIATION	DESCRIPTION
2D	Two-dimensional
3D	Three-dimensional
CompB	Composition B
C-J	Chapman-Jouguet
CSCM	Continuous surface cap model
FE	Finite element
FEA	Finite element analysis
FSI	Fluid structure interaction
HE	High explosive
1	Impulse density
JWL	Jones-Wilkins-Lee
N/A	Not applicable
Р	Pressure
PCCDN	Post crisis needs assessment tool in regards to construction damage and related needs
PETN	Pentaerythritol tetranitrate
RDX	Research department formula X (i.e. Cyclotrimetylenetrinitramine)
SOAP	Simple object access protocol
TNT	Trinitrotoluene

EXECUTIVE SUMMARY

This document is the first deliverable 'D3.1' of work package 3 in the RECONASS project, and it contains a description of the simulations performed to estimate blast impact on two different structures, as well as, a comparison of these results with experimental results obtained within RECONASS. The purpose of this work is to validate the simulation methodology, and also draw conclusions to be better prepared for the full scale final test of the RECONASS system.

In the work presented here, non-linear finite element (FE) simulations have been performed to analyse the response of the tested single members and a multi-node structure subjected to air blast. These were performed to determine the blast loads and the structural displacements prior to the model scale component testing. The FE simulations predicted the response of the components used in the model scale test with a good agreement, both regarding deformations and failure modes.

Simulations of the blasting of high explosives have been performed to predict the blast load on two types of structures. Structural analyses were then performed to predict the response of the reinforced concrete components to these blast loadings, for the instrumentation and setup of model scale tests.

The FE analyses have been verified by comparison with the obtained results from the model scale air blast high explosive testing of the reinforced concrete components. The FE analyses predicted the failure modes for the concrete structures. However, the magnitudes of the displacements were overestimated for the analyses of the slabs with three supports, and underestimated the deformations for reinforced concrete frame. Overall best performance was obtained for the FE analyses of the reinforced concrete frame, with a good agreement between the results from the component tests and from the FE analyses.

The results from the model scale reinforced concrete component testing, and the presented FE analyses, provide data for a reliable design of the setup for the full scale structural testing of the planned as the final test of the RECONASS system.

As an additional and separate task in this report, the development of algorithms correlating tag position before and after blast event have also been performed.